**DEPARTEMENT SAFETY STRUCTURES FIRE**

**Mechanical and Fire Resistance Division**

# REPORT No MRF 14 26049443/B

On

Fire Evaluation of

Post installed rebar connections

With Koelner Rawl R-KER injection system

**REQUESTED BY:**

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It comprises 30 pages numbered from 1/30 to 30/30

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# 1. TOPIC

When subjected to fire exposure, construction elements performances are reduced by the effect of the temperature increase. At the KOELNER RAWL company request, CSTB has performed a study aimed at the evaluation of the fire behaviour of the R-KER injection resin system used in conjunction with concrete reinforcing rebar (d 8 to 32 mm).

The present study is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature; neither does it deal with the design according to other accidental solicitations. This report presents values of bond capacities and load capacities respectively for an overlap joint application and for an anchorage application using the mortar product R-KER.

# 2. REFERENCES

[1] ETA-13/0805, RAWLPLUG S.A, R-KER / RV200, R-KER-W / RV200-W and R-KER-S / RV200-S, Bonded anchor with internal sleeves and rebars of sizes Ø8 to Ø32 for use in non-cracked concrete, 2013, INSTYTUT TECHNIKI BUDOWLANEJ

[2] TEST REPORT No MRF 14 26049443/A, Fire Testing of Post installed rebar connections with Koelner Rawl R-KER injection mortar, 2014, Centre Scientifique et Technique du Bâtiment

[3] CEN. EN 1991-1-2. Eurocode 1, Part 1-2: Actions on structures: general actions – actions on the structures exposed to fire. CEN, Bruxelles, Belgique; 2002.

[4] CEN. EN 1992-1-1. Eurocode 2, Part 1-1: Design of concrete structures - General rules and rules for buildings. CEN, Bruxelles, Belgique; 2005.

[5] CEN. EN 1992-1-2. Eurocode 2, Part 1-2: Design of concrete structures – General rules and structural fire design. CEN, Bruxelles, Belgique; 2005.

# 3. AUTHORS

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# 4. BACKROUND

## *4.1 Evaluation method*

The fire evaluation is performed with three steps.

1) First, an experimental program of pullout tests at high temperatures is carried out in order to determine a relationship between bond resistance and temperature [2]. This relationship is then expressed by a temperature reduction factor $0<k(\theta)<1$ which describes the decrease of resistance of the bond system (see PART 5).

2) Secondly, a thermal calculation using the method described in EN 1991-1-2, section 3 [3] is performed in order to determine the temperature distribution along the bonded rebar for each fire duration and for a given structural configuration.

3) Finally, at each time during the fire, the bond resistances are determined along the bonded rebar. For the anchor application the load resistance is calculated by integrating the bond resistances along the embedded depth.

Figure 1 presents the steps of the method used in this evaluation and the corresponding parts of the report.

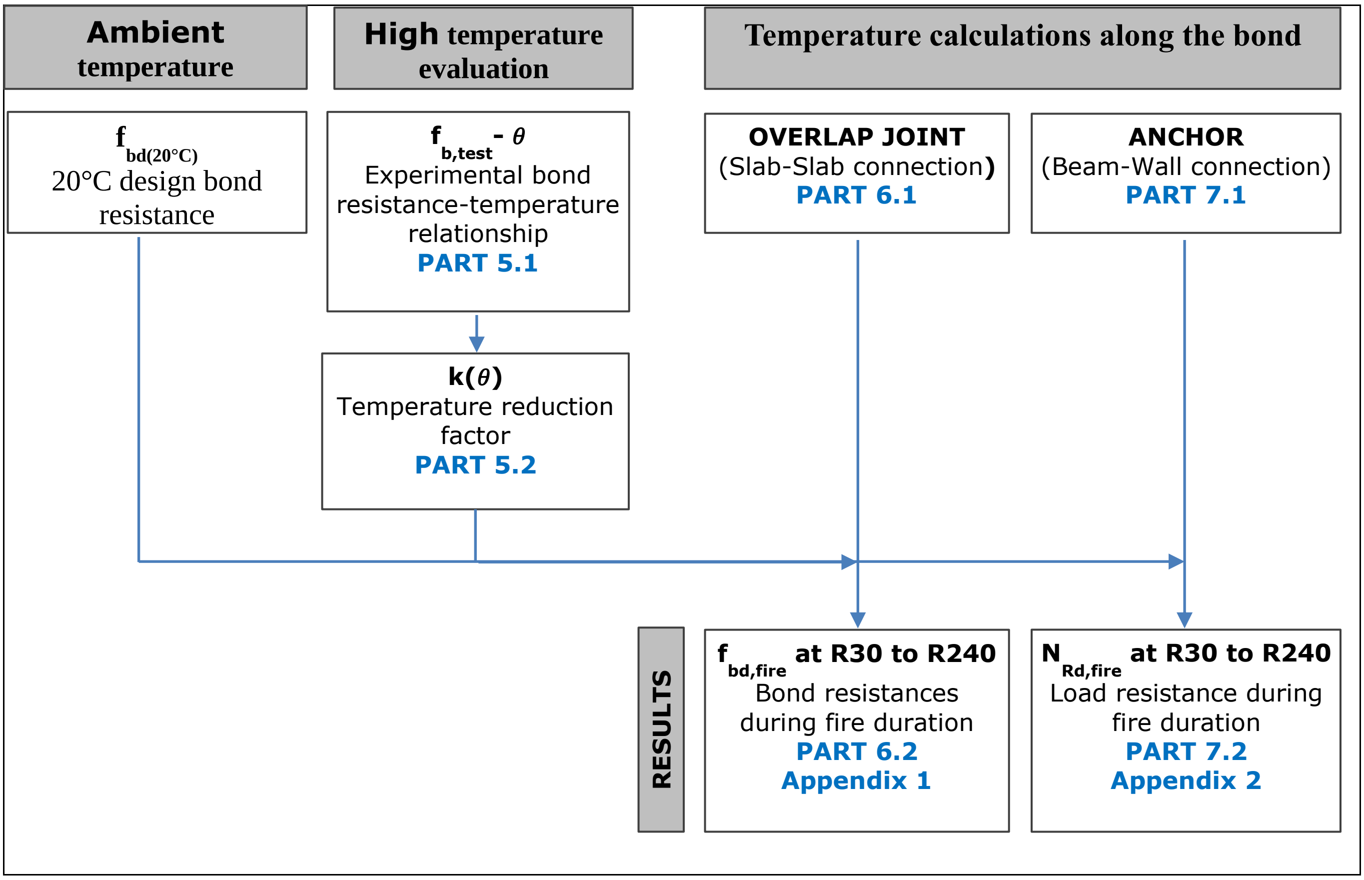



**Figure 1 : Method used for fire evaluation of bonded rebars**

The evaluation covers two structural uses of post-installed rebars in concrete (Figure 2): i) the overlap joint application and ii) the anchor application.

i) In the overlap joint application for a slab-slab configuration where the lower surface is exposed to fire, the temperature is uniform. The bond resistance is uniform along the bond and depends on the concrete cover and the duration of the fire (PART 6.2).

ii) In the anchor application for a beam-wall configuration where at least one side of the wall is exposed to fire, the temperature along the bond (inside the wall) is not uniform. This leads to different bond resistances and the load resistance is calculated by integration of the bond resistances along the lateral surface of the rebar (PART 7.2).

| OVERLAP JOINT (Slab-Slab connection) | ANCHOR (Beam-Wall connection) |
|---|---|

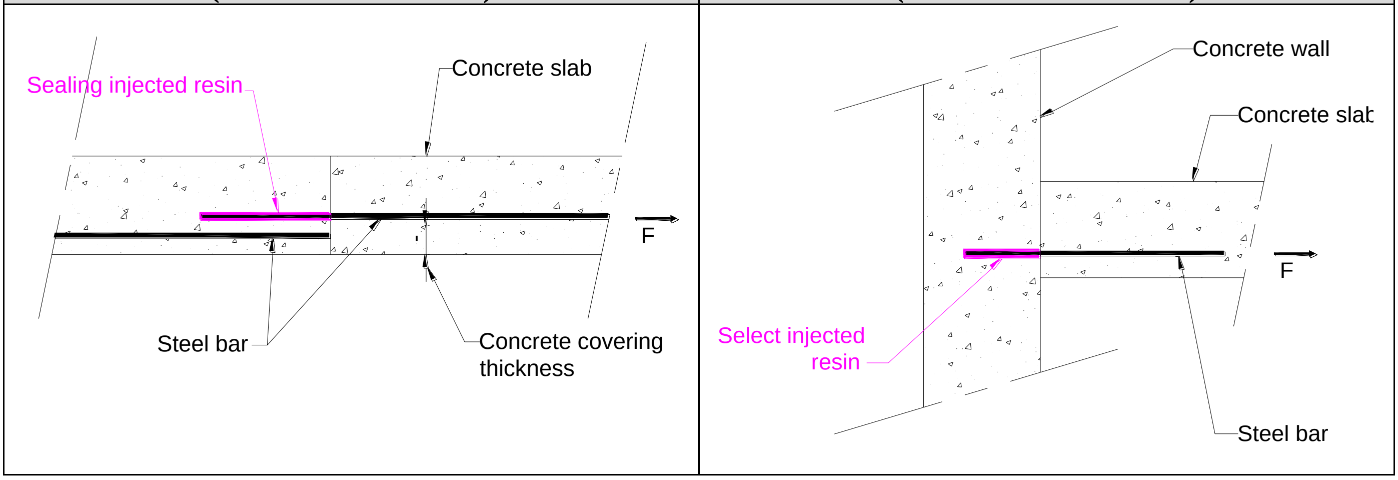



**Figure 2 : Sketches of a Slab-Slab connection left and of a Beam-Wall connection**

## 4.2 Application scope

The values of load resistances presented in this report are applicable for given parameters: Concrete class, structural configuration, fire duration, bar diameter, bond length, concrete cover and maximal temperatures. The result tables are provided in appendices 1 and 2.

**i) Concrete class**
The fire evaluation is applicable for concrete class C20/25 to C50/60. According to the ETA-12/0319 [1], the ultimate bond resistance in C20/25 concrete is equal to $f_{bd}$=2,30 N/mm² for bar diameters between 8 and 32 mm. The same bond resistance is conservatively considered for higher classes of concrete.

**ii) Structural configurations**
The fire evaluation covers slab-slab and beam-wall configurations for beams with a width higher than 40 cm. Load resistances of the beam-wall configuration may be conservatively applied to a slab-slab configuration. The bond resistances of the slab-slab configuration SHALL NOT be applied to a beam-beam configuration.

**iii) Fire durations**
The bond resistances and load resistances are provided at 30, 60, 90, 120, 180 and 240 min under a standardized ISO 834-1 fire. Thermal loading is calculated using the method described in EN 1991-1-2, section 3 [3].

**iv) Bar diameters**
The fire evaluation covers steel rebars with diameters of 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm with a yield strength of 500 N/mm².

**v)** **Bond lengths**

For the slab-slab configuration, the bond resistances are provided. The calculation of the bond length shall be carried out in accordance with ETA-12/0319 [1] and EN 1992-1-1, section 8 [4].

For the beam-wall connection, the load capacities are calculated for lengths between the minimal length $l_{b,min}$ (provided in the ETA-12/0319 [1]) and the maximal anchorage length conditioned either by installation restrictions ($l_{v,max}$ provided in the ETA-12/0319 [1]) or by the yielding of steel calculated in accordance with EN 1992-1-1, section 8 [4] (see equation below).

$l_{fire,min} = l_{b,min} = \max\{0{,}3.l_{b,rqd}\ ;\ 10.d\ ;\ 100\ mm\}$

Where $l_{b,rqd}$ is the required basic anchorage length

$$l_{b,rqd} = \frac{d}{4}.\frac{\sigma_{sd}}{f_{bd}} = \frac{d}{4}.\frac{\sigma_{s,yeild}}{\gamma_M.f_{bd}}$$

Where:

*$\sigma_{s,\ yield}$ = 500 N/mm² is the yield stress of steel*
*$\gamma_M$ = 1,5 is the material coefficient*
*$f_{bd}$ = 2.3 N/mm² is the design bond strength in C20/25 concrete.*
*d is the diameter of the bar*

$l_{fire,max} = \min\{l_{b,min}\ ;\ l_{steel\ yeilding}\}$

Where $l_{steel\ yielding}$ leads to the admissible load on the bond equal to:

$$N_{rebar\ yeild} = \frac{1}{\gamma_{M,20°C}}.\frac{\sigma_{s,yeild}}{\pi.(\frac{d}{2})^2}$$

Where:

*$\sigma_{s,\ yield}$ = 500 N/mm² is the yield stress of steel*
*$N_{rebar\ yield}$ is the design yielding load of the rebar*
*$\gamma_M$ = 1,5 is the material coefficient*
*d is the diameter of the bar*

Table 1 presents the maximal and minimal embedment lengths (established in the ETA-12/0319 [1]) and yielding loads.

**Table 1 : Maximal and minimal embedment lengths and yielding loads [1]**

| Rebar diameter (mm) | **8** | **10** | **12** | **14** | **16** | **20** | **25** | **28** | **32** |
|---|---|---|---|---|---|---|---|---|---|
| Drill diameter (mm) | 12 | 14 | 16 | 18 | 20 | 25 | 30 | 35 | 40 |
| Minimum anchorage length $l_{b,min}$ (mm) | 115 | 145 | 170 | 200 | 230 | 285 | 355 | 400 | 455 |
| Minimum anchorage length: overlap joint (mm) | 200 | 200 | 200 | 210 | 240 | 300 | 375 | 420 | 480 |
| Maximum anchorage depth $l_{v,max}$ (mm) | 400 | 500 | 600 | 700 | 800 | 1000 | 1000 | 1000 | 1000 |
| Design yielding load of the rebar (kN) | 37.7 | 51.3 | 67.0 | 84.8 | 105 | 164 | 236 | 321 | 419 |

**vi)** **Concrete cover**

Choice of the concrete cover shall be carried out in accordance with ETA-12/0319 [1] and EN 1992-1-1, section 4 [4]. In this evaluation, concrete cover is only considered for the thermal protection it brings to the rebar.

For the slab-slab configuration, bond resistances are provided for different concrete covers starting at 30 mm corresponding to the minimum concrete cover according to ETA-12/0319 [1].
For the beam-wall connection, the concrete cover in the beam influences the temperature distribution along the rebar in the thickness of the wall. The load resistances are provided for concrete covers inside the beam of 10, 20, 30, 40, 100 and 200 mm. Results are only provided for concrete covers superior to the diameter of the bar in accordance with EN 1992-1-1, section 4 [4].

**vii)** **Maximal temperatures**

In accordance to EN 1992-1-2, section 5 [5] steel resistance remains constant between 20°C and 350°C for bar laminated at high temperature. Therefore resistances are only considered along the parts of the bond below 350°C. Furthermore, the resistance is considered equal to zero above the temperature $\theta_{max}$ (described in PART 5.1) linked to the mortar behavior.

# 5. BOND RESISTANCE – TEMPERATURE RELATIONSHIP

## 5.1 Experimental bond resistances

A pullout test campaign is carried out at high temperature on 20 samples using rebars with a diameter of 12 mm and an embedment depth of 120 mm. The test procedure and results of this campaign are described in the test report n° MRF 14 26049443/A [2]. Table 2 presents the results of these tests.
The experimental bond resistances are determined directly from the applied load using the following equation.

$$f_b = \frac{N_{applied}}{\pi . d . l_v}$$

Where
$f_b$ is the bond resistance
$N_{applied}$ is the applied load during the heating
d is the bar diameter
$l_v$ is the embedment length

**Table 2 : Pullout test results performed at high temperatures**

| High temperature pullout test results | | | | | | | |
|---|---|---|---|---|---|---|---|
| Test n° | TC1 (°C) | TC2 (°C) | Bond Temperature (°C) | Load (kN) | d (mm) | $l_v$ (mm) | Bond resistance (N/mm²) |
| 1 | 42 | 67 | **54** | 48 | 12 | 120 | **10.6** |
| 2 | 52 | 76 | **64** | 36 | 12 | 120 | **7.9** |
| 3 | 52 | 81 | **67** | 42 | 12 | 120 | **9.2** |
| 4 | 254 | 357 | **306** | 2 | 12 | 120 | **0.4** |
| 5 | 65 | 88 | **77** | 31 | 12 | 120 | **6.8** |
| 6 | 119 | 169 | **144** | 7 | 12 | 120 | **1.6** |
| 7 | 90 | 98 | **94** | 25 | 12 | 120 | **5.5** |
| 8 | 139 | 132 | **135** | 12 | 12 | 120 | **2.7** |
| 9 | 97 | 100 | **98** | 20 | 12 | 120 | **4.4** |
| 10 | 134 | 100 | **117** | 17 | 12 | 120 | **3.8** |
| 11 | 37 | 44 | **41** | 45 | 12 | 120 | **9.9** |
| 12 | 63 | 100 | **82** | 33 | 12 | 120 | **7.3** |
| 13 | 24 | 27 | **26** | 52 | 12 | 120 | **11.5** |
| 14 | 269 | 224 | **246** | 4 | 12 | 120 | **0.9** |
| 15 | 88 | 97 | **92** | 28 | 12 | 120 | **6.2** |
| 16 | 191 | 136 | **163** | 6 | 12 | 120 | **1.3** |
| 17 | 140 | 209 | **175** | 5 | 12 | 120 | **1.1** |
| 18 | 153 | 122 | **138** | 15 | 12 | 120 | **3.3** |
| 19 | 87 | 80 | **84** | 41 | 12 | 120 | **9.0** |
| 20 | 52 | 72 | **62** | 50 | 12 | 120 | **11.1** |

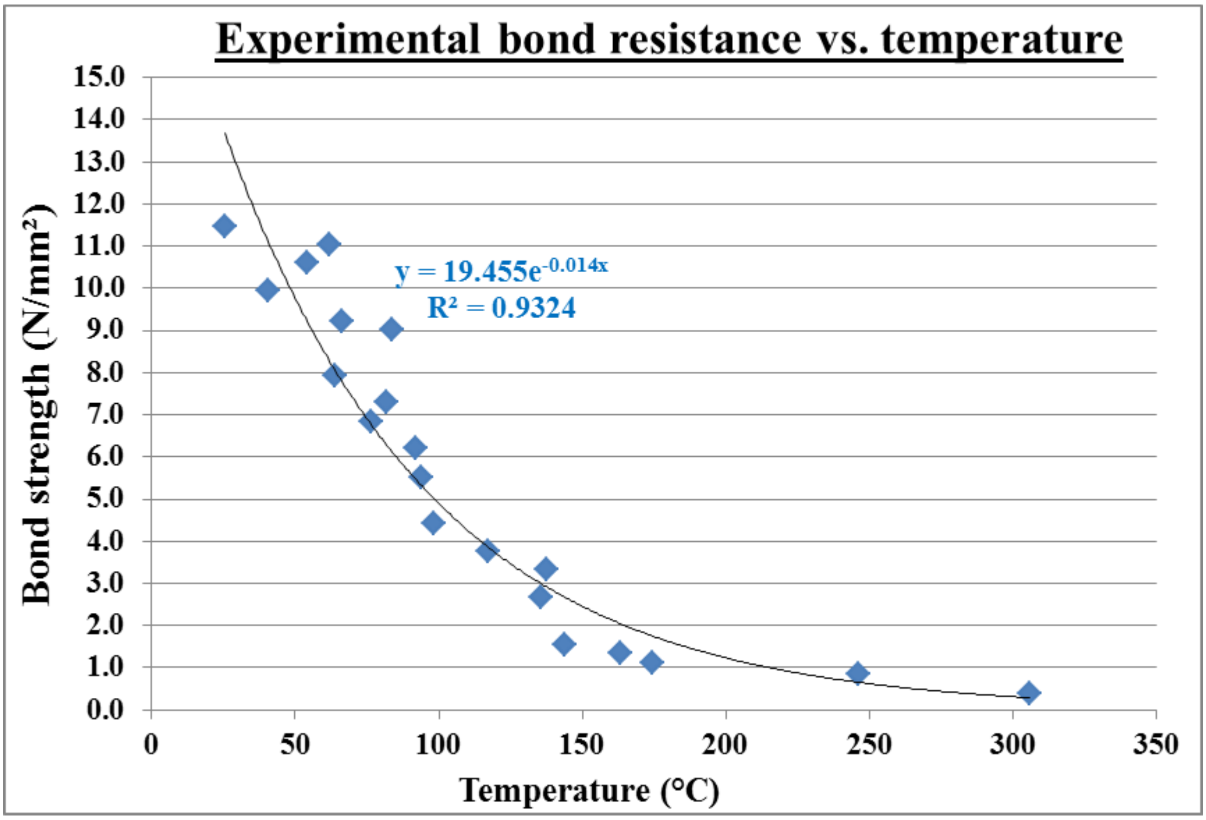



**Figure 3 : Experimental bond strengths vs temperature and trend curve**

The load values were chosen during the test campaign to ensure that the maximal distance between two data points was lower than 1,5 N/mm² and lower than 100°C between 20°C and 306°C. Figure 3 presents the experimental bond resistances vs. temperature. An exponential trend curve is used to describe the bond resistance-temperature relationship analytically using the fallowing equation.

$$f_{bm}(\theta) = a.e^{-b.\theta}$$

Where:
$f_{bm}(\theta)$ *is the* mean bond resistance at the temperature θ (in N/mm²)
θ is the temperature of the bond material
a and b are the exponential fitting curve constants

The maximal temperature reached during the tests is identified as $\theta_{max}$.

For the KOELNER RAWL R-KER injection system, the a, b and $\theta_{max}$ parameters are presented in Table 3.

**Table 3 : Injection system parameters**

| $f_{bm}$-$\theta$ parameters | | |
|---|---|---|
| a= | 19.455 | N/mm² |
| b= | 0.014 | /°C |
| $\theta$ max= | 306 | °C |

## 5.2 Temperature reduction factor

The temperature reduction factor k(θ) is determined from the fitted curve $f_{bm}(\theta)$ to describe the variation of resistance of the injection system with temperature. It is calculated using the fallowing equations.

$$k(\theta) = \frac{f_{bm}(\theta)}{f_{bm,req,d}} \leq 1 \quad for \quad 20°C \leq \theta \leq \theta_{max}$$

$$k(\theta) = 0 \quad for \quad \theta > \theta_{max}$$

Where:
*k(θ)* temperature reduction factor
$f_{bm}(\theta)$ *is* the mean bond resistance at the temperature θ
$f_{bm,req,d}$ *=min{10 N/mm² ;* $f_{bm}(\theta)$*} is the* required bond resistance at cold state
*θ is the* temperature of the bond
$\theta_{max}$ maximal temperature measured during the tests

Figure 4 presents the variation of the temperature reduction factor vs. temperature for the R-KEM injection system.
No extrapolation beyond test temperatures is allowed. For temperatures higher than the maximal measured temperature during the tests ($\theta_{max}$), the reduction factor k(θ) is equal to zero.

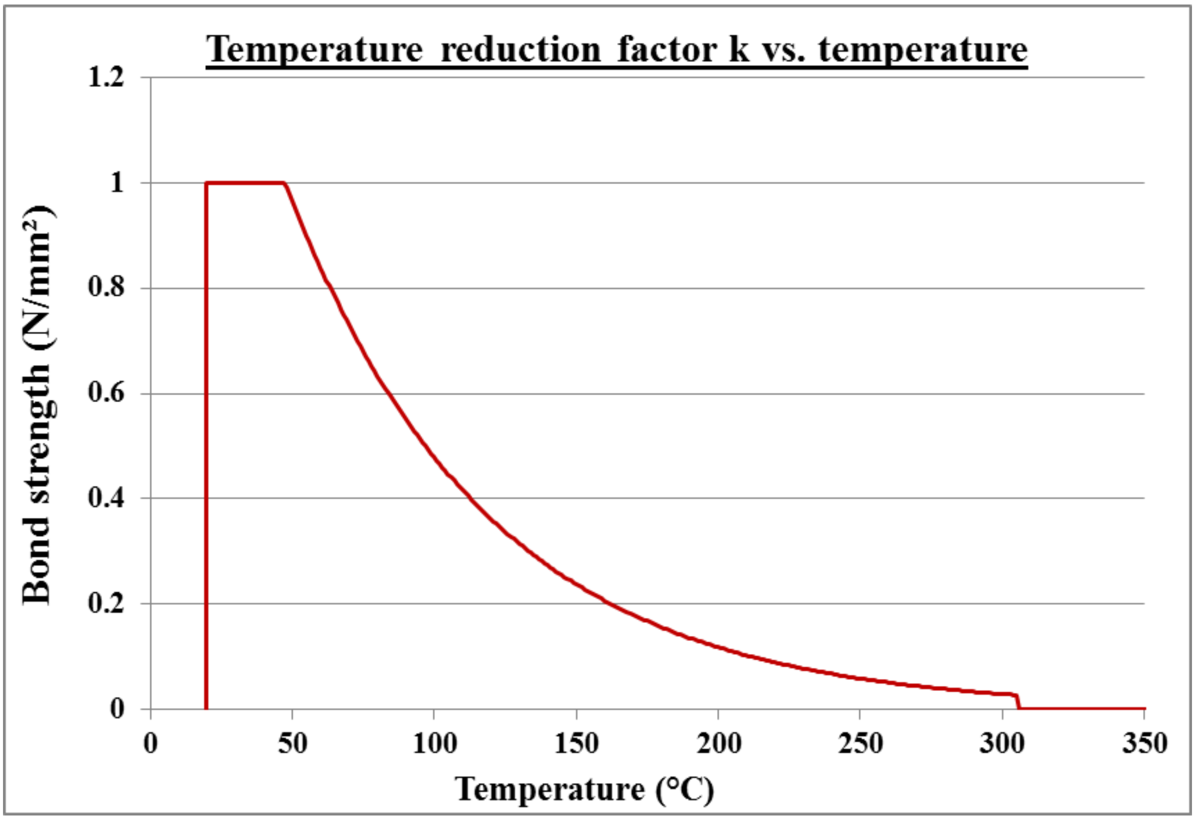



**Figure 4 : Temperature reduction factor k(θ) vs. temperature**

# 6. OVERLAP JOINT APPLICATION (SLAB-SLAB CONNECTION)

## *6.1 Temperature fields*

The knowledge of the fire behaviour of traditional concrete structures to assess the temperature distribution for every fire duration by modeling the thermal exchanges inside concrete elements. The temperature profile depends on the connection configuration: slab-slab or beam-wall. These temperatures are calculated using the finite elements method in accordance with EN 1991-1-2, section 3 [3] with the CAST3M software.

At the initial time (t=0) every element temperature is supposed equal to 20°C.

The fire is modeled by a heat flux on the exposed faces of the structure. This heat flux is a function of the gas temperature $\theta_g$ for which the evolution is given by the conventional ISO 834-1 time-temperature relationship (EN 1991-1-2, section 3 [3]).

$$\theta_g(t) = \theta_0 + 345.\log_{10}(8.t + 1)$$

Where:

$\theta_g$ is the gas temperature
$\theta_0$=20°C is the initial temperature
t is the time in minutes

The entering flux in a heated element is the sum of the convective and the radiation parts:

- convective flux density: $\varphi_c = h.(\theta_g - \theta_s)$ (W/m²),
- radiation flux density: $\varphi_c = \varepsilon.\sigma.(\theta_g^4 - \theta_s^4)$ (W/m²).

Where:

$\sigma$ is the Stefan-Boltzmann parameter
$\theta_s$ is the surface temperature of the heated element
$\varepsilon$ is the resulting emissive coefficient
$h$ is the exchange coefficient for convection

The exchange coefficients, presented in Table 4, are given by EN 1992-1-2, appendix A [5].

**Table 4 : Values for the exchange coefficients**

| | h(W/m²K) | $\varepsilon$ |
|---|---|---|
| **Fire exposed side** | **25** | ***0.7*** |

In this study, only concrete is considered in the thermal calculation (EN 1992-1-2, section 4 [5]). The concrete thermal properties are provided by EN 1992-1-2, section 3 [5]. The variations of thermal conductivity, mass density and specific heat are represented in Figure 5. The peak of the specific heat corresponds to a concrete having a water percentage of 1,5% in accordance with EN 1992-1-2, appendix A [5].

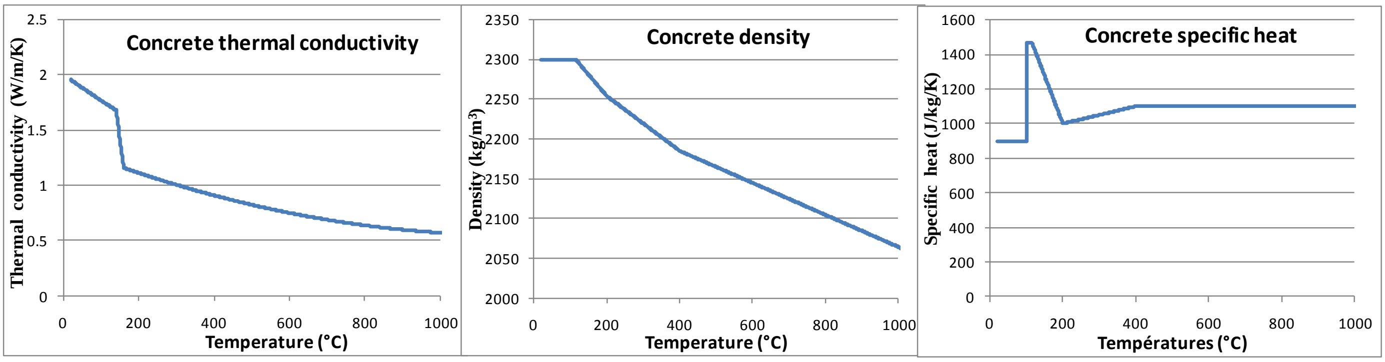



**Figure 5 : Variations of thermal conductivity, density and specific heat of concrete according to EN 1992-1-2**

For a slab-slab connection (Figure 2), the thermal calculation is carried out on a two dimensional mesh by applying the fire heat flux as boundary condition on the lower surface. No boundary condition at 20°C is applied on the upper surface to be conservative.

The isotherms are horizontal implying that the temperature is uniform along the bonding interface and equal to the temperature in a slab at a depth equivalent to the concrete cover. Figure 6 presents the temperature versus concrete cover at 0, 30, 60, 90, 120, 180 and 240 min during an ISO 834-1 fire. The same temperature curves are provided in EN 1992-1-2, appendix A [5].

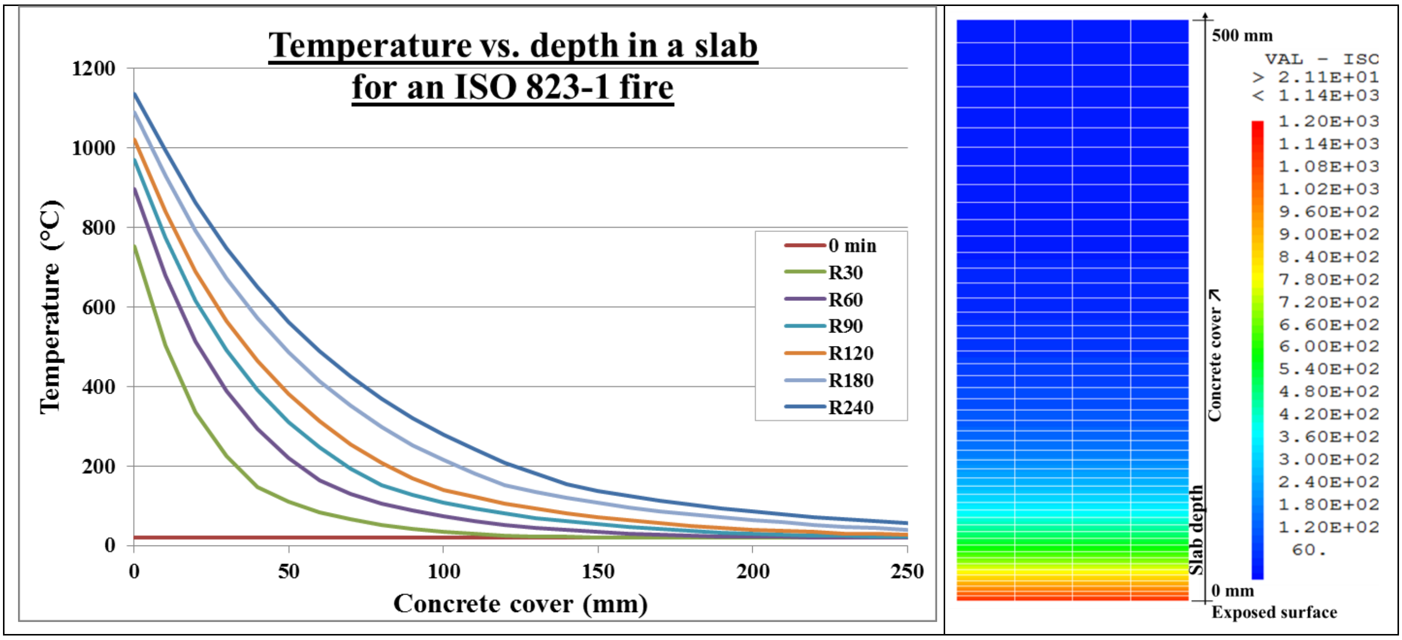



**Figure 6 : Temperature vs. concrete cover temperature at 0, 30, 60, 90, 120, 180 and 240 min during an ISO 834-1 fire**

## 6.2 Design bond resistances

From the temperature curves (Part 6.1, Figure 6) and the temperature reduction factor k($\theta$) (Part 5.2, Figure 4), the values of the design bond resistances $f_{bd,fire}$ are determined using the fallowing equation.

$$f_{bd,fire}(\theta) = f_{bd,20°C} \cdot \frac{\gamma_{M,20°C}}{\gamma_{M,fire}} . k(\theta)$$

Where:

$f_{bd,fire}(\theta)$ is the design bond resistance that depends on temperature
$f_{bd,20°C}$=2,3 for C20/25 concrete is the design bond strength at 20°C
$\gamma_{M,20°C}$=1,5 is the material coefficient at ambient temperature
$\gamma_{M,fire}$=1 is the material coefficient in a fire situation
k($\theta$) is the temperature reduction factor

Appendix 1 presents values of the design bond resistance for different concrete covers at 30, 60, 90, 120, 180 and 240 min during an ISO 834-1 fire.

The material safety factor applicable for the accidental situation of fire is equal to 1 according to EN 1992-1-2, section 2 [5], while it is equal to 1,5 at ambient temperature. This leads to obtaining higher values of load resistances at the beginning of a fire in fire design in comparison to ambient temperature design for the same rebar geometry. Design at ambient temperature shall be carried out before fire design.

# 7. ANCHOR APPLICATION (BEAM-WALL CONNECTION)

## *7.1 Temperature fields*

For a beam-wall connection (Figure 2) where the rebar is bonded inside the wall, there is a temperature gradient in the thickness of the wall. The temperature along the bonding interface is not uniform and depends on the fire duration, the anchoring length and the concrete cover of the rebar inside the beam (which acts as a protection against thermal exposure). Therefore, the temperature profiles along the bond are determined for each fire duration, for each bonded length and for the concrete covers inside the beam of 10, 20, 30 and 40 mm.

A three dimensional mesh was used. Due to symmetry considerations only half of the structure is meshed (Figure 7). The same calculation parameters (material thermal properties, time-temperature curve, convective and radiation exchange coefficients) as the ones described in PART 6.1 are applied.

The boundary conditions are:

- On the lower and lateral sides of the beam fire heat fluxes are applied to the elements.
- On the side of the wall where the beam is connected, the fire heat fluxes are applied to the elements.
- No heat exchange condition is applied on the other sides.

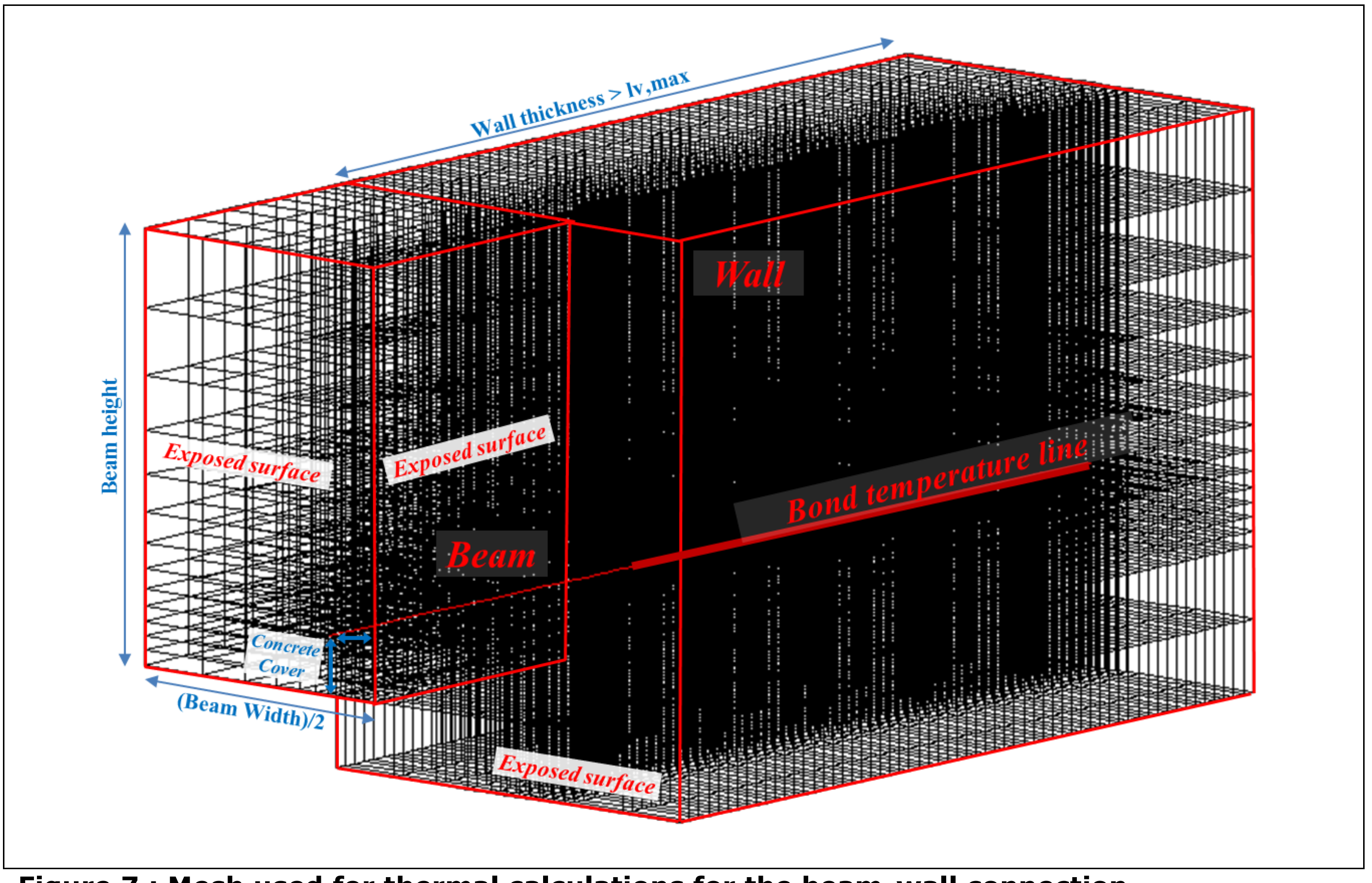



**Figure 7 : Mesh used for thermal calculations for the beam-wall connection**

Figure 8 presents the calculated thermal fields at 30, 90 and 240 min. The geometry of the mesh of the beam used for calculations is taken large enough so that the isotherms at 240 min of heating are parallel to the concrete surfaces (Figure 8). This implies that the same temperature profiles along the rebar would be obtained for larger and higher beams. The beam height was equal to 300 mm and the beam width was equal to 400 mm.

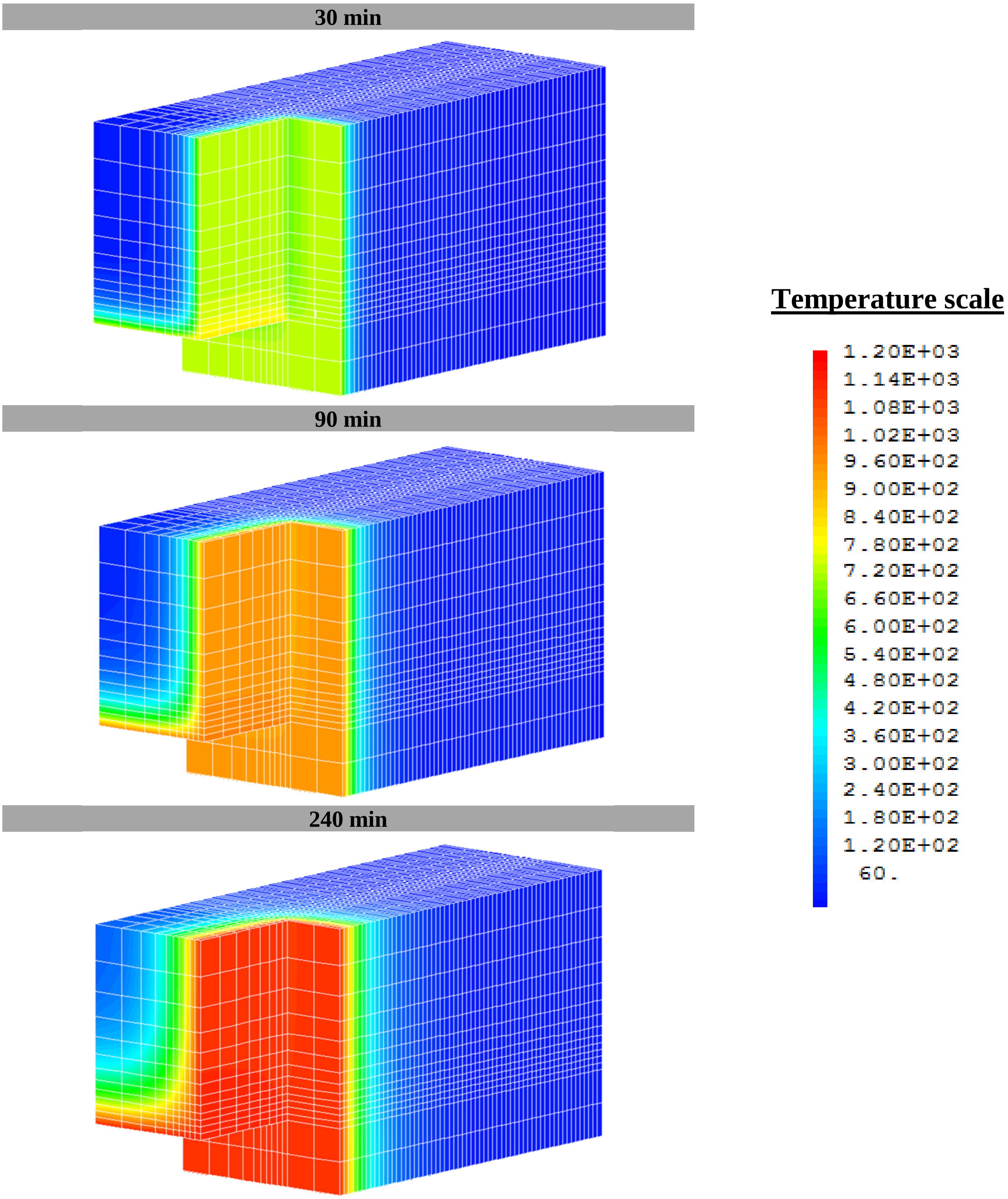



**Figure 8 : Temperature fields at 30, 90 and 240 min during an ISO 834-1 fire for the beam-wall connection**

## 7.2 Design load resistances

From the calculated temperature profiles and from the temperature reduction factor k($\theta$) (Part 5.2, Figure 4), the values of design load capacities $N_{Rd,fire}$ are determined by integration of the design bond resistances.

$$N_{Rd,fire} = \pi.d.\int_0^{l_v} f_{bd,fire}(\theta(x)).dx = \pi.d.f_{bd,20°C}.\frac{\gamma_{M,20°C}}{\gamma_{M,fire}}.\int_0^{l_v} k(\theta(x)).dx$$

Where:
$N_{Rd,fire}$ is the design load resistance at a given time during the fire
$f_{bd,20°C}$=2,3 N/mm² is the design bond strength at 20°C
$\gamma_{M,20°C}$=1,5 is the material coefficient at ambient temperature
$\gamma_{M,fire}$=1 is the material coefficient in a fire situation
k($\theta$) is the temperature reduction factor
$l_v$ is the embedment depth of the bonded rebar
The integration is performed by finite differences using the following equation.

$$N_{Rd,fire} \approx \pi.d.f_{bd,20°C}.\frac{\gamma_{M,20°C}}{\gamma_{M,fire}}.\sum_0^{l_v} k(\theta_i).\Delta x$$

For the calculation, the value of $\Delta$x was taken equal to 10 mm and the maximal temperature reduction factor k($\theta_i$) on the length of $\Delta$x was taken into account.

Figure 9 presents a general example (not from the R-KER mortar) of the calculation of the design load resistance by integration of $f_{bd}$ on a bond length of 250 mm by using the temperature profile along the bond at 120 min during an ISO 834-1 fire with a concrete cover of 20 mm in the beam.

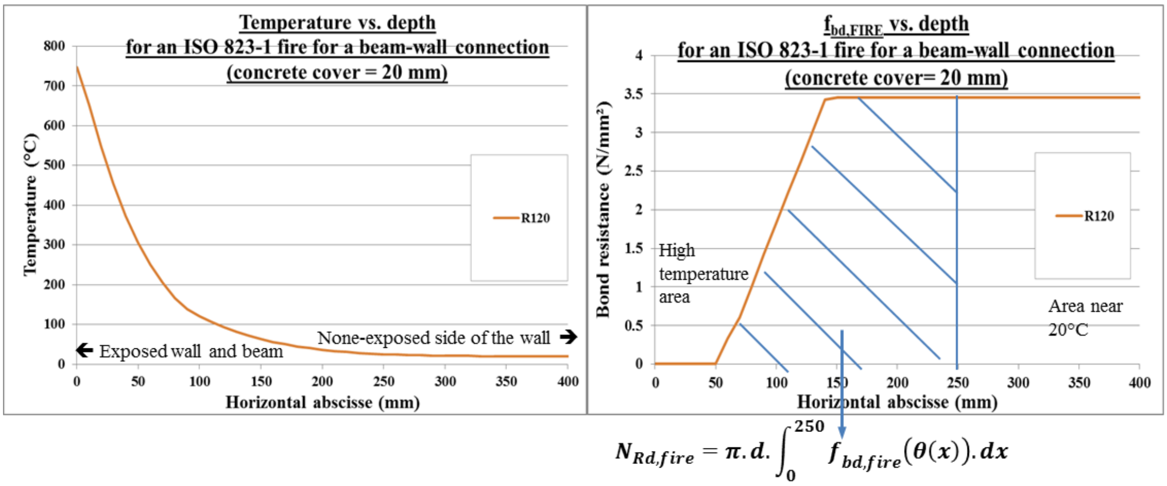



**Figure 9 : General example of the calculation of the design load resistance by integration of $f_{bd}$**

Appendices 2.1, 2.2, 2.3, 2.4, 2.5 and 2.6 present the values of $N_{Rd,fire}$ at different fire durations for different bond lengths respectively for concrete covers of 10 mm, 20 mm, 30 mm, 40 mm, 100 mm and 200 mm. The minimal and maximal values of bond lengths are in accordance with PART 4.2.

# 8. LIST OF APPENDICES

**Appendix 1**: Design bond resistances for an overlap joint application (slab-slab connection)

**Appendix 2.1**: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 10 mm for diameters 8 and 10 mm

**Appendix 2.2**: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 20 mm for diameters 8, 10, 12, 14, 16 and 20 mm

**Appendix 2.3**: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 30 mm for diameters 8, 10, 12, 14, 16, 20, 25 and 28 mm

**Appendix 2.4**: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 40 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

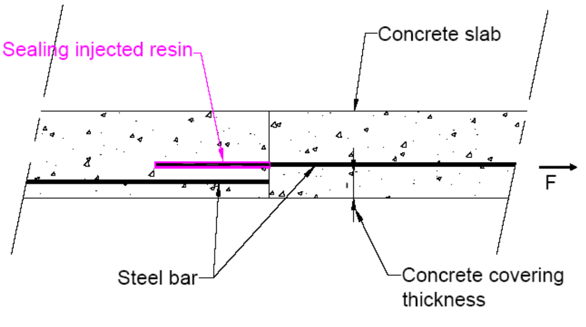
**Appendix 2.5**: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 100 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

**Appendix 2.6**: Design load resistances for an anchoring application (beam-wall connection) with a concrete cover of 200 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

## *Appendix 1:*
## Maximum applicable bond stress for an overlap joint application

The table presents design bond resistances ($f_{bd}$) for a **Slab-Slab connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for concrete covers between 30 and 230 mm.

The bond resistance values shall not be applied for beam-beam connections. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



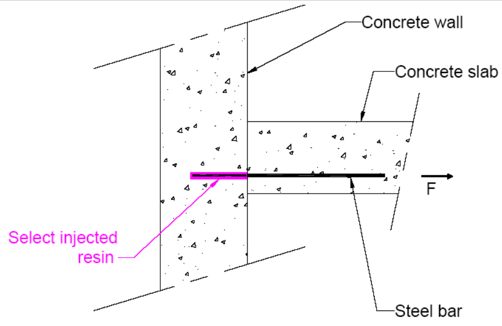

| Fire Design Bond Resistance $f_{bd,FIRE}$ (N/mm²) | | | | | | |
|---|---|---|---|---|---|---|
| **Concrete Cover (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **20** | | | | | | |
| **30** | 0.3 | | | | | |
| **40** | 0.9 | 0.1 | | | | |
| **50** | 1.4 | 0.3 | | | | |
| **60** | 2.0 | 0.7 | 0.2 | | | |
| **70** | 2.7 | 1.1 | 0.4 | 0.2 | | |
| **80** | 3.2 | 1.5 | 0.8 | 0.4 | 0.1 | |
| **90** | 3.5 | 1.9 | 1.1 | 0.6 | 0.2 | |
| **100** | | 2.4 | 1.5 | 0.9 | 0.3 | 0.1 |
| **110** | | 2.8 | 1.8 | 1.2 | 0.5 | 0.2 |
| **120** | | 3.2 | 2.2 | 1.5 | 0.8 | 0.4 |
| **130** | | 3.5 | 2.5 | 1.8 | 1.0 | 0.5 |
| **140** | | | 2.9 | 2.1 | 1.3 | 0.8 |
| **150** | | | 3.2 | 2.4 | 1.5 | 1.0 |
| **160** | | | 3.5 | 2.7 | 1.7 | 1.2 |
| **170** | | | | 3.0 | 2.0 | 1.4 |
| **180** | | | | 3.3 | 2.2 | 1.6 |
| **190** | | | | 3.5 | 2.5 | 1.8 |
| **200** | | | | | 2.7 | 2.0 |
| **210** | | | | | 3.0 | 2.2 |
| **220** | | | | | 3.2 | 2.4 |
| **230** | | | | | 3.4 | 2.7 |
| **240** | | | | | 3.5 | 2.9 |
| **250** | | | | | | 3.1 |
| **260** | | | | | | 3.3 |
| **270** | | | | | | 3.4 |
| **280** | | | | | | 3.5 |

*The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*

## *Appendix 2.1:*
## Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 10 mm for diameters 8 and 10 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 10 mm** and for diameters 8 and 10 mm.

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.



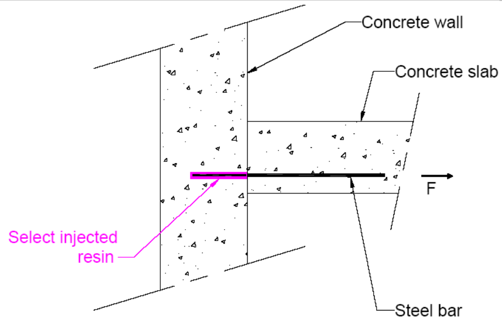

| Concrete Cover = 10 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 8 | 120 | 5.7 | 3.2 | 1.9 | 1.1 | 0.5 | 0.2 |
| | 150 | 8.3 | 5.8 | 3.9 | 2.7 | 1.4 | 0.8 |
| | 180 | 10.9 | 8.4 | 6.5 | 4.9 | 2.9 | 1.8 |
| | 210 | 13.5 | 11.0 | 9.1 | 7.5 | 5.0 | 3.4 |
| | 250 | 16.8 | 14.5 | 12.6 | 11.0 | 8.4 | 6.2 |
| | 280 | | 16.8 | 15.2 | 13.6 | 11.0 | 8.7 |
| | 300 | | | 16.8 | 15.3 | 12.7 | 10.5 |
| | 320 | | | | 16.8 | 14.4 | 12.2 |
| | 350 | | | | | 16.8 | 14.8 |
| | 380 | | | | | | 16.8 |
| 10 | 150 | 10.4 | 7.3 | 4.9 | 3.4 | 1.8 | 1.0 |
| | 180 | 13.7 | 10.5 | 8.2 | 6.2 | 3.6 | 2.3 |
| | 220 | 18.0 | 14.9 | 12.5 | 10.5 | 7.2 | 5.0 |
| | 260 | 22.4 | 19.2 | 16.8 | 14.8 | 11.5 | 8.8 |
| | 300 | 26.2 | 23.5 | 21.2 | 19.2 | 15.9 | 13.1 |
| | 330 | | 26.2 | 24.4 | 22.4 | 19.1 | 16.3 |
| | 350 | | | 26.2 | 24.6 | 21.3 | 18.5 |
| | 370 | | | | 26.2 | 23.5 | 20.7 |
| | 400 | | | | | 26.2 | 23.9 |
| | 430 | | | | | | 26.2 |

*Calculations are carried out taking the minimal concrete cover (CSTB report n°26048096). Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*

## *Appendix 2.2:*
## Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 20 mm for diameters 8, 10, 12, 14, 16 and 20 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 20 mm** and for diameters 8, 10, 12, 14, 16 and 20 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.




| Concrete Cover = 20 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **8** | **120** | 6.1 | 3.5 | 2.0 | 1.2 | 0.5 | 0.2 |
| | **160** | 9.6 | 7.0 | 5.0 | 3.5 | 2.0 | 1.1 |
| | **200** | 13.0 | 10.4 | 8.5 | 6.9 | 4.4 | 2.9 |
| | **250** | 16.8 | 14.8 | 12.8 | 11.2 | 8.6 | 6.3 |
| | **280** | | 16.8 | 15.4 | 13.8 | 11.2 | 8.9 |
| | **300** | | | 16.8 | 15.5 | 12.9 | 10.6 |
| | **320** | | | | 16.8 | 14.6 | 12.4 |
| | **350** | | | | | 16.8 | 15.0 |
| | **380** | | | | | | 16.8 |
| **10** | **150** | 10.9 | 7.6 | 5.2 | 3.6 | 1.9 | 1.0 |
| | **200** | 16.3 | 13.0 | 10.6 | 8.6 | 5.5 | 3.6 |
| | **250** | 21.7 | 18.5 | 16.0 | 14.0 | 10.7 | 7.9 |
| | **300** | 26.2 | 23.9 | 21.5 | 19.4 | 16.1 | 13.3 |
| | **330** | | 26.2 | 24.7 | 22.7 | 19.4 | 16.6 |
| | **350** | | | 26.2 | 24.9 | 21.5 | 18.7 |
| | **370** | | | | 26.2 | 23.7 | 20.9 |
| | **400** | | | | | 26.2 | 24.1 |
| | **420** | | | | | | 26.2 |

*The table continues on the next page.*

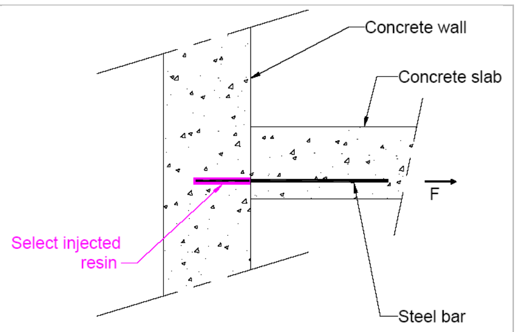
| Concrete Cover = 20 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **12** | **170** | 15.6 | 11.7 | 8.8 | 6.5 | 3.7 | 2.2 |
| | **200** | 19.5 | 15.6 | 12.7 | 10.3 | 6.6 | 4.4 |
| | **240** | 24.7 | 20.9 | 17.9 | 15.5 | 11.5 | 8.3 |
| | **290** | 31.2 | 27.4 | 24.4 | 22.0 | 18.0 | 14.7 |
| | **340** | 37.7 | 33.9 | 30.9 | 28.5 | 24.6 | 21.2 |
| | **370** | | 37.7 | 34.8 | 32.4 | 28.5 | 25.1 |
| | **400** | | | 37.7 | 36.3 | 32.4 | 29.0 |
| | **420** | | | | 37.7 | 35.0 | 31.6 |
| | **450** | | | | | 37.7 | 35.5 |
| | **470** | | | | | | 37.7 |
| **14** | **200** | 22.8 | 18.3 | 14.9 | 12.0 | 7.7 | 5.1 |
| | **240** | 28.9 | 24.3 | 20.9 | 18.1 | 13.5 | 9.7 |
| | **280** | 34.9 | 30.4 | 27.0 | 24.2 | 19.5 | 15.6 |
| | **330** | 42.5 | 38.0 | 34.6 | 31.8 | 27.1 | 23.2 |
| | **390** | 51.3 | 47.1 | 43.7 | 40.9 | 36.2 | 32.3 |
| | **420** | | 51.3 | 48.2 | 45.4 | 40.8 | 36.8 |
| | **450** | | | 51.3 | 50.0 | 45.3 | 41.4 |
| | **460** | | | | 51.3 | 46.9 | 42.9 |
| | **490** | | | | | 51.3 | 47.5 |
| | **520** | | | | | | 51.3 |
| **16** | **230** | 31.2 | 26.1 | 22.2 | 19.0 | 13.7 | 9.6 |
| | **280** | 39.9 | 34.7 | 30.9 | 27.6 | 22.3 | 17.8 |
| | **330** | 48.6 | 43.4 | 39.5 | 36.3 | 31.0 | 26.5 |
| | **380** | 57.3 | 52.1 | 48.2 | 45.0 | 39.7 | 35.2 |
| | **440** | 67.0 | 62.5 | 58.6 | 55.4 | 50.1 | 45.6 |
| | **470** | | 67.0 | 63.8 | 60.6 | 55.3 | 50.8 |
| | **490** | | | 67.0 | 64.0 | 58.7 | 54.2 |
| | **510** | | | | 67.0 | 62.2 | 57.7 |
| | **540** | | | | | 67.0 | 62.9 |
| | **570** | | | | | | 67.0 |
| **20** | **290** | 52.1 | 45.6 | 40.7 | 36.7 | 30.1 | 24.4 |
| | **340** | 62.9 | 56.4 | 51.6 | 47.5 | 40.9 | 35.3 |
| | **400** | 75.9 | 69.4 | 64.6 | 60.5 | 53.9 | 48.3 |
| | **460** | 88.9 | 82.4 | 77.6 | 73.6 | 66.9 | 61.3 |
| | **540** | 104.7 | 99.8 | 94.9 | 90.9 | 84.3 | 78.6 |
| | **570** | | 104.7 | 101.4 | 97.4 | 90.8 | 85.1 |
| | **590** | | | 104.7 | 101.7 | 95.1 | 89.5 |
| | **610** | | | | 104.7 | 99.4 | 93.8 |
| | **640** | | | | | 104.7 | 100.3 |
| | **670** | | | | | | 104.7 |

*Calculations are carried out taking the minimal concrete cover (CSTB report n°26048096). Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*

## *Appendix 2.3:*
## Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 30 mm for diameters 8, 10, 12, 14, 16, 20, 25 and 28 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 30 mm** and for diameters 8, 10, 12, 14, 16, 20, 25 and 28 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.




| Concrete Cover = 30 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **8** | **120** | 6.6 | 3.8 | 2.2 | 1.4 | 0.6 | 0.3 |
| | **150** | 9.2 | 6.4 | 4.5 | 3.1 | 1.6 | 0.9 |
| | **190** | 12.7 | 9.9 | 7.9 | 6.3 | 3.9 | 2.5 |
| | **240** | 16.8 | 14.2 | 12.3 | 10.6 | 7.9 | 5.8 |
| | **270** | | 16.8 | 14.9 | 13.2 | 10.5 | 8.3 |
| | **300** | | | 16.8 | 15.8 | 13.1 | 10.9 |
| | **320** | | | | 16.8 | 14.9 | 12.6 |
| | **350** | | | | | 16.8 | 15.2 |
| | **370** | | | | | | 16.8 |
| **10** | **150** | 11.6 | 8.0 | 5.6 | 3.9 | 2.0 | 1.2 |
| | **190** | 15.9 | 12.4 | 9.9 | 7.9 | 4.8 | 3.2 |
| | **240** | 21.3 | 17.8 | 15.3 | 13.3 | 9.9 | 7.2 |
| | **290** | 26.2 | 23.2 | 20.8 | 18.7 | 15.3 | 12.5 |
| | **320** | | 26.2 | 24.0 | 22.0 | 18.6 | 15.8 |
| | **350** | | | 26.2 | 25.2 | 21.8 | 19.0 |
| | **360** | | | | 26.2 | 22.9 | 20.1 |
| | **400** | | | | | 26.2 | 24.4 |
| | **420** | | | | | | 26.2 |

*The table continues on the next page.*

| Concrete Cover = 30 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 12 | 170 | 16.5 | 12.2 | 9.3 | 6.9 | 3.9 | 2.4 |
| | 200 | 20.4 | 16.2 | 13.2 | 10.7 | 6.9 | 4.6 |
| | 240 | 25.6 | 21.4 | 18.4 | 15.9 | 11.9 | 8.6 |
| | 280 | 30.8 | 26.6 | 23.6 | 21.2 | 17.1 | 13.7 |
| | 340 | 37.7 | 34.4 | 31.4 | 29.0 | 24.9 | 21.5 |
| | 370 | | 37.7 | 35.3 | 32.9 | 28.8 | 25.4 |
| | 390 | | | 37.7 | 35.5 | 31.4 | 28.0 |
| | 410 | | | | 37.7 | 34.0 | 30.6 |
| | 430 | | | | | 36.6 | 33.2 |
| | 440 | | | | | 37.7 | 34.5 |
| | 470 | | | | | | 37.7 |
| 14 | 200 | 23.8 | 18.8 | 15.4 | 12.5 | 8.0 | 5.4 |
| | 240 | 29.8 | 24.9 | 21.5 | 18.6 | 13.9 | 10.1 |
| | 280 | 35.9 | 31.0 | 27.5 | 24.7 | 19.9 | 16.0 |
| | 320 | 42.0 | 37.1 | 33.6 | 30.7 | 26.0 | 22.1 |
| | 390 | 51.3 | 47.7 | 44.2 | 41.4 | 36.6 | 32.7 |
| | 420 | | 51.3 | 48.8 | 45.9 | 41.2 | 37.2 |
| | 440 | | | 51.3 | 49.0 | 44.2 | 40.3 |
| | 460 | | | | 51.3 | 47.2 | 43.3 |
| | 490 | | | | | 51.3 | 47.9 |
| | 520 | | | | | | 51.3 |
| 16 | 230 | 32.4 | 26.7 | 22.8 | 19.5 | 14.1 | 10.0 |
| | 270 | 39.3 | 33.7 | 29.7 | 26.5 | 21.0 | 16.5 |
| | 310 | 46.2 | 40.6 | 36.7 | 33.4 | 28.0 | 23.5 |
| | 360 | 54.9 | 49.3 | 45.3 | 42.1 | 36.7 | 32.2 |
| | 430 | 67.0 | 61.4 | 57.5 | 54.2 | 48.8 | 44.3 |
| | 470 | | 67.0 | 64.4 | 61.2 | 55.7 | 51.2 |
| | 490 | | | 67.0 | 64.6 | 59.2 | 54.7 |
| | 510 | | | | 67.0 | 62.7 | 58.2 |
| | 540 | | | | | 67.0 | 63.4 |
| | 570 | | | | | | 67.0 |
| 20 | 290 | 53.5 | 46.4 | 41.5 | 37.4 | 30.6 | 25.0 |
| | 340 | 64.3 | 57.3 | 52.3 | 48.3 | 41.5 | 35.9 |
| | 390 | 75.1 | 68.1 | 63.2 | 59.1 | 52.3 | 46.7 |
| | 460 | 90.3 | 83.3 | 78.4 | 74.3 | 67.5 | 61.9 |
| | 530 | 104.7 | 98.5 | 93.5 | 89.4 | 82.7 | 77.0 |
| | 560 | | 104.7 | 100.0 | 95.9 | 89.2 | 83.6 |
| | 590 | | | 104.7 | 102.5 | 95.7 | 90.1 |
| | 610 | | | | 104.7 | 100.0 | 94.4 |
| | 640 | | | | | 104.7 | 100.9 |
| | 660 | | | | | | 104.7 |

*The table continues on the next page.*

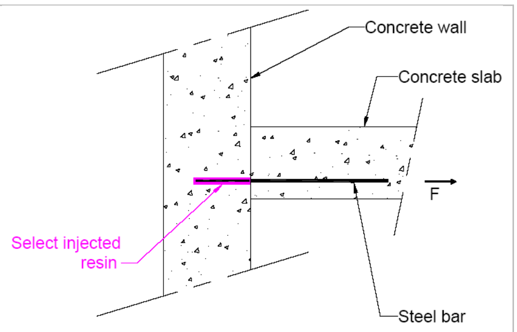
| Concrete Cover = 30 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 25 | 360 | 85.8 | 77.0 | 70.8 | 65.7 | 57.3 | 50.2 |
| | 410 | 99.3 | 90.6 | 84.4 | 79.3 | 70.8 | 63.8 |
| | 470 | 115.6 | 106.8 | 100.7 | 95.6 | 87.1 | 80.1 |
| | 530 | 131.9 | 123.1 | 116.9 | 111.8 | 103.3 | 96.3 |
| | 580 | 145.4 | 136.6 | 130.5 | 125.4 | 116.9 | 109.9 |
| | 650 | 163.6 | 155.6 | 149.4 | 144.3 | 135.9 | 128.8 |
| | 680 | | 163.6 | 157.6 | 152.5 | 144.0 | 137.0 |
| | 710 | | | 163.6 | 160.6 | 152.1 | 145.1 |
| | 730 | | | | 163.6 | 157.5 | 150.5 |
| | 760 | | | | | 163.6 | 158.6 |
| | 780 | | | | | | 163.6 |
| 28 | 400 | 108.2 | 98.4 | 91.5 | 85.8 | 76.3 | 68.4 |
| | 460 | 126.4 | 116.6 | 109.7 | 104.0 | 94.5 | 86.6 |
| | 520 | 144.6 | 134.8 | 127.9 | 122.2 | 112.7 | 104.8 |
| | 580 | 162.9 | 153.0 | 146.1 | 140.4 | 130.9 | 123.0 |
| | 650 | 184.1 | 174.3 | 167.4 | 161.6 | 152.2 | 144.3 |
| | 720 | 205.3 | 195.5 | 188.6 | 182.9 | 173.4 | 165.5 |
| | 760 | | 205.3 | 200.7 | 195.0 | 185.5 | 177.7 |
| | 780 | | | 205.3 | 201.1 | 191.6 | 183.7 |
| | 800 | | | | 205.3 | 197.7 | 189.8 |
| | 830 | | | | | 205.3 | 198.9 |
| | 860 | | | | | | 205.3 |

*Calculations are carried out taking the minimal concrete cover (CSTB report n°26048096). Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*

## *Appendix 2.4:*
## Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 40 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 40 mm** and for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.




| Concrete Cover = 40 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **8** | **120** | 7.4 | 4.3 | 2.6 | 1.6 | 0.7 | 0.3 |
| | **150** | 10.0 | 6.9 | 4.9 | 3.4 | 1.8 | 1.0 |
| | **180** | 12.6 | 9.6 | 7.5 | 5.8 | 3.5 | 2.2 |
| | **230** | 16.8 | 13.9 | 11.8 | 10.1 | 7.3 | 5.2 |
| | **270** | | 16.8 | 15.3 | 13.6 | 10.8 | 8.5 |
| | **290** | | | 16.8 | 15.3 | 12.6 | 10.3 |
| | **310** | | | | 16.8 | 14.3 | 12.0 |
| | **340** | | | | | 16.8 | 14.6 |
| | **370** | | | | | | 16.8 |
| **10** | **150** | 12.5 | 8.7 | 6.1 | 4.2 | 2.3 | 1.3 |
| | **180** | 15.7 | 11.9 | 9.3 | 7.2 | 4.3 | 2.7 |
| | **230** | 21.2 | 17.4 | 14.7 | 12.6 | 9.2 | 6.5 |
| | **280** | 26.2 | 22.8 | 20.2 | 18.1 | 14.6 | 11.7 |
| | **320** | | 26.2 | 24.5 | 22.4 | 18.9 | 16.1 |
| | **340** | | | 26.2 | 24.6 | 21.1 | 18.2 |
| | **360** | | | | 26.2 | 23.3 | 20.4 |
| | **390** | | | | | 26.2 | 23.7 |
| | **420** | | | | | | 26.2 |

*The table continues on the next page.*

| Concrete Cover = 40 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 12 | 170 | 17.6 | 13.0 | 9.9 | 7.4 | 4.3 | 2.6 |
| | 200 | 21.5 | 16.9 | 13.8 | 11.3 | 7.3 | 4.9 |
| | 240 | 26.7 | 22.1 | 19.0 | 16.5 | 12.3 | 9.0 |
| | 280 | 31.9 | 27.3 | 24.2 | 21.7 | 17.5 | 14.1 |
| | 330 | 37.7 | 33.8 | 30.7 | 28.2 | 24.0 | 20.6 |
| | 360 | | 37.7 | 34.6 | 32.1 | 27.9 | 24.5 |
| | 390 | | | 37.7 | 36.0 | 31.8 | 28.4 |
| | 410 | | | | 37.7 | 34.4 | 31.0 |
| | 440 | | | | | 37.7 | 34.9 |
| | 470 | | | | | | 37.7 |
| 14 | 200 | 25.1 | 19.7 | 16.1 | 13.1 | 8.5 | 5.7 |
| | 240 | 31.1 | 25.8 | 22.1 | 19.2 | 14.4 | 10.5 |
| | 280 | 37.2 | 31.9 | 28.2 | 25.3 | 20.4 | 16.4 |
| | 330 | 44.8 | 39.5 | 35.8 | 32.9 | 28.0 | 24.0 |
| | 380 | 51.3 | 47.1 | 43.4 | 40.5 | 35.6 | 31.6 |
| | 410 | | 51.3 | 47.9 | 45.0 | 40.2 | 36.1 |
| | 440 | | | 51.3 | 49.6 | 44.7 | 40.7 |
| | 460 | | | | 51.3 | 47.8 | 43.7 |
| | 490 | | | | | 51.3 | 48.3 |
| | 510 | | | | | | 51.3 |
| 16 | 230 | 33.9 | 27.8 | 23.6 | 20.2 | 14.7 | 10.4 |
| | 270 | 40.8 | 34.7 | 30.5 | 27.2 | 21.6 | 17.0 |
| | 320 | 49.5 | 43.4 | 39.2 | 35.8 | 30.3 | 25.7 |
| | 380 | 59.9 | 53.8 | 49.6 | 46.2 | 40.7 | 36.1 |
| | 430 | 67.0 | 62.5 | 58.3 | 54.9 | 49.4 | 44.8 |
| | 460 | | 67.0 | 63.5 | 60.1 | 54.6 | 50.0 |
| | 490 | | | 67.0 | 65.3 | 59.8 | 55.2 |
| | 500 | | | | 67.0 | 61.5 | 56.9 |
| | 540 | | | | | 67.0 | 63.9 |
| | 560 | | | | | | 67.0 |
| 20 | 290 | 55.3 | 47.7 | 42.5 | 38.3 | 31.4 | 25.6 |
| | 340 | 66.2 | 58.6 | 53.3 | 49.1 | 42.2 | 36.5 |
| | 390 | 77.0 | 69.4 | 64.2 | 60.0 | 53.1 | 47.3 |
| | 450 | 90.0 | 82.4 | 77.2 | 73.0 | 66.1 | 60.3 |
| | 520 | 104.7 | 97.6 | 92.3 | 88.1 | 81.2 | 75.5 |
| | 560 | | 104.7 | 101.0 | 96.8 | 89.9 | 84.2 |
| | 580 | | | 104.7 | 101.2 | 94.2 | 88.5 |
| | 600 | | | | 104.7 | 98.6 | 92.8 |
| | 630 | | | | | 104.7 | 99.3 |
| | 660 | | | | | | 104.7 |

*The table continues on the next page.*

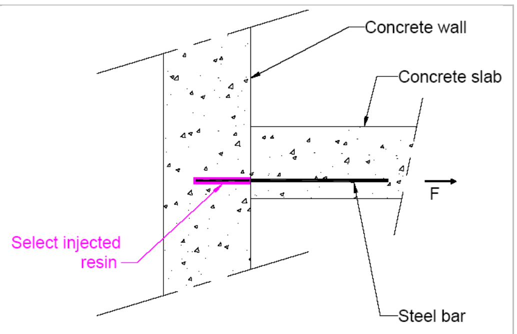
| Concrete Cover = 40 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 25 | 360 | 88.1 | 78.6 | 72.1 | 66.8 | 58.2 | 51.0 |
| | 410 | 101.7 | 92.2 | 85.6 | 80.4 | 71.7 | 64.6 |
| | 470 | 117.9 | 108.4 | 101.9 | 96.6 | 88.0 | 80.8 |
| | 530 | 134.2 | 124.7 | 118.1 | 112.9 | 104.3 | 97.1 |
| | 580 | 147.7 | 138.2 | 131.7 | 126.4 | 117.8 | 110.6 |
| | 640 | 163.6 | 154.5 | 147.9 | 142.7 | 134.1 | 126.9 |
| | 680 | | 163.6 | 158.8 | 153.5 | 144.9 | 137.7 |
| | 700 | | | 163.6 | 159.0 | 150.3 | 143.1 |
| | 720 | | | | 163.6 | 155.7 | 148.5 |
| | 750 | | | | | 163.6 | 156.7 |
| | 780 | | | | | | 163.6 |
| 28 | 400 | 110.9 | 100.2 | 92.9 | 87.0 | 77.3 | 69.3 |
| | 460 | 129.1 | 118.4 | 111.1 | 105.2 | 95.5 | 87.5 |
| | 530 | 150.3 | 139.6 | 132.3 | 126.4 | 116.8 | 108.7 |
| | 600 | 171.5 | 160.9 | 153.5 | 147.7 | 138.0 | 130.0 |
| | 660 | 189.8 | 179.1 | 171.8 | 165.9 | 156.2 | 148.2 |
| | 720 | 205.3 | 197.3 | 190.0 | 184.1 | 174.4 | 166.4 |
| | 750 | | 205.3 | 199.1 | 193.2 | 183.5 | 175.5 |
| | 780 | | | 205.3 | 202.3 | 192.6 | 184.6 |
| | 790 | | | | 205.3 | 195.7 | 187.6 |
| | 830 | | | | | 205.3 | 199.8 |
| | 850 | | | | | | 205.3 |
| 32 | 460 | 147.5 | 135.3 | 126.9 | 120.2 | 109.2 | 100.0 |
| | 530 | 171.8 | 159.6 | 151.2 | 144.5 | 133.4 | 124.2 |
| | 610 | 199.5 | 187.3 | 179.0 | 172.3 | 161.2 | 152.0 |
| | 680 | 223.8 | 211.6 | 203.2 | 196.5 | 185.5 | 176.3 |
| | 740 | 244.6 | 232.4 | 224.0 | 217.3 | 206.3 | 197.1 |
| | 810 | 268.1 | 256.7 | 248.3 | 241.6 | 230.6 | 221.4 |
| | 850 | | 268.1 | 262.2 | 255.5 | 244.4 | 235.2 |
| | 870 | | | 268.1 | 262.4 | 251.4 | 242.2 |
| | 890 | | | | 268.1 | 258.3 | 249.1 |
| | 920 | | | | | 268.1 | 259.5 |
| | 950 | | | | | | 268.1 |

*Calculations are carried out taking the minimal concrete cover (CSTB report n°26048096). Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*

## *Appendix 2.5:*
## Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 100 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 100 mm** and for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.




| Concrete Cover = 100 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **8** | **120** | 10.4 | 8.9 | 6.6 | 4.7 | 2.5 | 1.3 |
| | **160** | 13.9 | 12.4 | 10.1 | 7.9 | 4.7 | 2.9 |
| | **200** | 16.8 | 15.9 | 13.5 | 11.4 | 7.8 | 5.3 |
| | **220** | | 16.8 | 15.3 | 13.1 | 9.6 | 6.8 |
| | **240** | | | 16.8 | 14.9 | 11.3 | 8.4 |
| | **270** | | | | 16.8 | 13.9 | 11.0 |
| | **310** | | | | | 16.8 | 14.5 |
| | **340** | | | | | | 16.8 |
| **10** | **150** | 16.3 | 14.4 | 11.5 | 8.8 | 5.1 | 3.1 |
| | **200** | 21.7 | 19.8 | 16.9 | 14.2 | 9.8 | 6.6 |
| | **250** | 26.2 | 25.3 | 22.3 | 19.7 | 15.2 | 11.6 |
| | **260** | | 26.2 | 23.4 | 20.8 | 16.3 | 12.7 |
| | **290** | | | 26.2 | 24.0 | 19.5 | 16.0 |
| | **320** | | | | 26.2 | 22.8 | 19.2 |
| | **360** | | | | | 26.2 | 23.5 |
| | **390** | | | | | | 26.2 |

*The table continues on the next page.*

| Concrete Cover = 100 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 12 | 170 | 22.1 | 19.9 | 16.4 | 13.2 | 8.1 | 5.2 |
| | 210 | 27.3 | 25.1 | 21.6 | 18.4 | 13.0 | 9.0 |
| | 250 | 32.5 | 30.3 | 26.8 | 23.6 | 18.2 | 13.9 |
| | 290 | 37.7 | 35.5 | 32.0 | 28.8 | 23.4 | 19.1 |
| | 310 | | 37.7 | 34.6 | 31.4 | 26.0 | 21.7 |
| | 340 | | | 37.7 | 35.3 | 29.9 | 25.6 |
| | 360 | | | | 37.7 | 32.5 | 28.2 |
| | 400 | | | | | 37.7 | 33.4 |
| | 440 | | | | | | 37.7 |
| 14 | 200 | 30.3 | 27.8 | 23.7 | 19.9 | 13.7 | 9.3 |
| | 240 | 36.4 | 33.9 | 29.7 | 26.0 | 19.8 | 14.7 |
| | 290 | 44.0 | 41.4 | 37.3 | 33.6 | 27.3 | 22.3 |
| | 340 | 51.3 | 49.0 | 44.9 | 41.2 | 34.9 | 29.9 |
| | 360 | | 51.3 | 47.9 | 44.2 | 38.0 | 33.0 |
| | 390 | | | 51.3 | 48.8 | 42.5 | 37.5 |
| | 410 | | | | 51.3 | 45.6 | 40.5 |
| | 450 | | | | | 51.3 | 46.6 |
| | 490 | | | | | | 51.3 |
| 16 | 230 | 39.9 | 37.0 | 32.3 | 28.0 | 20.8 | 15.2 |
| | 270 | 46.8 | 43.9 | 39.2 | 34.9 | 27.8 | 22.1 |
| | 330 | 57.2 | 54.3 | 49.6 | 45.3 | 38.2 | 32.5 |
| | 390 | 67.0 | 64.7 | 60.0 | 55.7 | 48.6 | 42.9 |
| | 410 | | 67.0 | 63.5 | 59.2 | 52.1 | 46.3 |
| | 440 | | | 67.0 | 64.4 | 57.3 | 51.5 |
| | 460 | | | | 67.0 | 60.7 | 55.0 |
| | 500 | | | | | 67.0 | 61.9 |
| | 530 | | | | | | 67.0 |
| 20 | 290 | 62.9 | 59.2 | 53.3 | 48.0 | 39.1 | 31.9 |
| | 340 | 73.7 | 70.0 | 64.2 | 58.8 | 49.9 | 42.7 |
| | 390 | 84.5 | 80.9 | 75.0 | 69.7 | 60.7 | 53.6 |
| | 450 | 97.5 | 93.9 | 88.0 | 82.7 | 73.7 | 66.6 |
| | 490 | 104.7 | 102.6 | 96.7 | 91.4 | 82.4 | 75.3 |
| | 500 | | 104.7 | 98.8 | 93.5 | 84.6 | 77.4 |
| | 530 | | | 104.7 | 100.0 | 91.1 | 83.9 |
| | 560 | | | | 104.7 | 97.6 | 90.4 |
| | 600 | | | | | 104.7 | 99.1 |
| | 630 | | | | | | 104.7 |

*The table continues on the next page.*

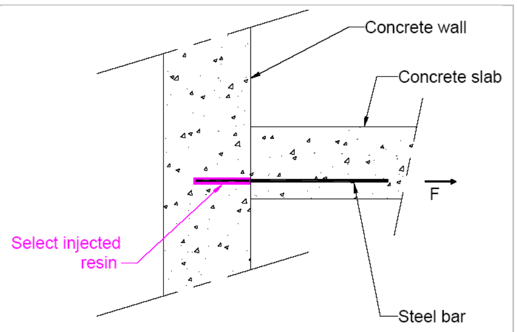
| Concrete Cover = 100 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **25** | **360** | 97.5 | 93.0 | 85.6 | 79.0 | 67.8 | 58.8 |
| | **410** | 111.1 | 106.5 | 99.2 | 92.5 | 81.3 | 72.4 |
| | **480** | 130.1 | 125.5 | 118.1 | 111.5 | 100.3 | 91.4 |
| | **550** | 149.0 | 144.5 | 137.1 | 130.5 | 119.3 | 110.3 |
| | **610** | 163.6 | 160.7 | 153.4 | 146.7 | 135.5 | 126.6 |
| | **630** | | 163.6 | 158.8 | 152.1 | 141.0 | 132.0 |
| | **650** | | | 163.6 | 157.6 | 146.4 | 137.4 |
| | **680** | | | | 163.6 | 154.5 | 145.6 |
| | **720** | | | | | 163.6 | 156.4 |
| | **750** | | | | | | 163.6 |
| **28** | **400** | 121.4 | 116.3 | 108.0 | 100.6 | 88.1 | 78.0 |
| | **460** | 139.6 | 134.5 | 126.2 | 118.8 | 106.3 | 96.3 |
| | **530** | 160.8 | 155.7 | 147.5 | 140.0 | 127.5 | 117.5 |
| | **610** | 185.1 | 180.0 | 171.8 | 164.3 | 151.8 | 141.8 |
| | **680** | 205.3 | 201.2 | 193.0 | 185.6 | 173.0 | 163.0 |
| | **700** | | 205.3 | 199.1 | 191.6 | 179.1 | 169.1 |
| | **730** | | | 205.3 | 200.7 | 188.2 | 178.2 |
| | **750** | | | | 205.3 | 194.3 | 184.3 |
| | **790** | | | | | 205.3 | 196.4 |
| | **820** | | | | | | 205.3 |
| **32** | **460** | 159.5 | 153.7 | 144.3 | 135.8 | 121.5 | 110.0 |
| | **540** | 187.3 | 181.4 | 172.0 | 163.5 | 149.2 | 137.8 |
| | **630** | 218.5 | 212.6 | 203.2 | 194.7 | 180.4 | 169.0 |
| | **710** | 246.3 | 240.4 | 231.0 | 222.5 | 208.2 | 196.7 |
| | **780** | 268.1 | 264.7 | 255.3 | 246.8 | 232.5 | 221.0 |
| | **790** | | 268.1 | 258.7 | 250.2 | 235.9 | 224.5 |
| | **820** | | | 268.1 | 260.6 | 246.3 | 234.9 |
| | **850** | | | | 268.1 | 256.7 | 245.3 |
| | **890** | | | | | 268.1 | 259.1 |
| | **920** | | | | | | 268.1 |

*Calculations are carried out taking the minimal concrete cover (CSTB report n°26048096). Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*

## *Appendix 2.6:*
## Maximum applicable loads for an anchoring application (beam-wall connection) with a concrete cover of 200 mm for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The table presents design load resistances for a **Beam-Wall connection** using **C20/25 concrete** and rebars with a yield strength **$f_y$=500 N/mm²** in an **ISO 834-1 fire** (at 30, 60, 90, 120, 180 and 240 min) for a **concrete cover of 200 mm** and for diameters 8, 10, 12, 14, 16, 20, 25, 28 and 32 mm

The design load values may be used safely for a slab-wall connection. Post-installed rebars shall be designed in ambient temperature conditions before being designed in fire conditions.




| Concrete Cover = 200 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| **Diameter (mm)** | **Length lv (mm)** | **R30** | **R60** | **R90** | **R120** | **R180** | **R240** |
| **8** | **120** | 10.4 | 10.4 | 10.4 | 10.2 | 7.4 | 5.2 |
| | **150** | 13.0 | 13.0 | 13.0 | 12.8 | 10.0 | 7.1 |
| | **180** | 15.6 | 15.6 | 15.6 | 15.4 | 12.6 | 9.4 |
| | **200** | 16.8 | 16.8 | 16.8 | 16.8 | 14.3 | 11.1 |
| | **230** | | | | | 16.8 | 13.7 |
| | **270** | | | | | | 16.8 |
| **10** | **150** | 16.3 | 16.3 | 16.3 | 16.0 | 12.5 | 8.9 |
| | **180** | 19.5 | 19.5 | 19.5 | 19.3 | 15.7 | 11.8 |
| | **210** | 22.8 | 22.8 | 22.8 | 22.5 | 19.0 | 15.0 |
| | **250** | 26.2 | 26.2 | 26.2 | 26.2 | 23.3 | 19.3 |
| | **280** | | | | | 26.2 | 22.5 |
| | **320** | | | | | | 26.2 |
| **12** | **170** | 22.1 | 22.1 | 22.1 | 21.8 | 17.6 | 12.9 |
| | **200** | 26.0 | 26.0 | 26.0 | 25.8 | 21.5 | 16.6 |
| | **240** | 31.2 | 31.2 | 31.2 | 31.0 | 26.7 | 21.9 |
| | **290** | 37.7 | 37.7 | 37.7 | 37.5 | 33.2 | 28.4 |
| | **300** | | | | 37.7 | 34.5 | 29.7 |
| | **330** | | | | | 37.7 | 33.6 |
| | **370** | | | | | | 37.7 |

*The table continues on the next page.*

| Concrete Cover = 200 mm | | Fire Design Load Resistance $N_{Rd,fire}$ (kN) | | | | | |
|---|---|---|---|---|---|---|---|
| Diameter (mm) | Length lv (mm) | R30 | R60 | R90 | R120 | R180 | R240 |
| 14 | 200 | 30.3 | 30.3 | 30.3 | 30.0 | 25.1 | 19.4 |
| | 240 | 36.4 | 36.4 | 36.4 | 36.1 | 31.1 | 25.5 |
| | 290 | 44.0 | 44.0 | 44.0 | 43.7 | 38.7 | 33.1 |
| | 340 | 51.3 | 51.3 | 51.3 | 51.3 | 46.3 | 40.7 |
| | 350 | | | | 51.3 | 47.8 | 42.2 |
| | 380 | | | | | 51.3 | 46.7 |
| | 420 | | | | | | 51.3 |
| 16 | 230 | 39.9 | 39.9 | 39.9 | 39.5 | 33.8 | 27.4 |
| | 270 | 46.8 | 46.8 | 46.8 | 46.5 | 40.8 | 34.3 |
| | 320 | 55.5 | 55.5 | 55.5 | 55.1 | 49.4 | 43.0 |
| | 390 | 67.0 | 67.0 | 67.0 | 67.0 | 61.6 | 55.1 |
| | 430 | | | | | 67.0 | 62.1 |
| | 460 | | | | | | 67.0 |
| 20 | 290 | 62.9 | 62.9 | 62.9 | 62.4 | 55.3 | 47.3 |
| | 340 | 73.7 | 73.7 | 73.7 | 73.3 | 66.1 | 58.1 |
| | 390 | 84.5 | 84.5 | 84.5 | 84.1 | 77.0 | 68.9 |
| | 450 | 97.5 | 97.5 | 97.5 | 97.1 | 90.0 | 81.9 |
| | 490 | 104.7 | 104.7 | 104.7 | 104.7 | 98.7 | 90.6 |
| | 520 | | | | | 104.7 | 97.1 |
| | 560 | | | | | | 104.7 |
| 25 | 360 | 97.5 | 97.5 | 97.5 | 97.0 | 88.1 | 78.0 |
| | 410 | 111.1 | 111.1 | 111.1 | 110.6 | 101.6 | 91.6 |
| | 470 | 127.4 | 127.4 | 127.4 | 126.8 | 117.9 | 107.8 |
| | 540 | 146.3 | 146.3 | 146.3 | 145.8 | 136.9 | 126.8 |
| | 610 | 163.6 | 163.6 | 163.6 | 163.6 | 155.8 | 145.8 |
| | 640 | | | | | 163.6 | 153.9 |
| | 680 | | | | | | 163.6 |
| 28 | 400 | 121.4 | 121.4 | 121.4 | 120.8 | 110.8 | 99.5 |
| | 460 | 139.6 | 139.6 | 139.6 | 139.0 | 129.0 | 117.7 |
| | 530 | 160.8 | 160.8 | 160.8 | 160.2 | 150.3 | 139.0 |
| | 610 | 185.1 | 185.1 | 185.1 | 184.5 | 174.5 | 163.3 |
| | 680 | 205.3 | 205.3 | 205.3 | 205.3 | 195.8 | 184.5 |
| | 720 | | | | | 205.3 | 196.7 |
| | 750 | | | | | | 205.3 |
| 32 | 460 | 159.5 | 159.5 | 159.5 | 158.8 | 147.4 | 134.6 |
| | 530 | 183.8 | 183.8 | 183.8 | 183.1 | 171.7 | 158.8 |
| | 610 | 211.6 | 211.6 | 211.6 | 210.9 | 199.5 | 186.6 |
| | 680 | 235.8 | 235.8 | 235.8 | 235.1 | 223.7 | 210.9 |
| | 740 | 256.7 | 256.7 | 256.7 | 256.0 | 244.6 | 231.7 |
| | 780 | 268.1 | 268.1 | 268.1 | 268.1 | 258.4 | 245.6 |
| | 810 | | | | | 268.1 | 256.0 |
| | 850 | | | | | | 268.1 |

*Calculations are carried out taking the minimal concrete cover (CSTB report n°26048096). Intermediate values may be interpolated linearly. Extrapolation is not possible. The present table is aimed at supplying data for the design of the injection anchoring system when exposed to fire. This study does not deal with the mechanical design at ambient temperature, neither does it deal with the design according to other accidental solicitations, these shall be done in addition.*